# Downstream propagation of water and sediment hydrographs due to the hypothetical failure of a real earthen dam



# Introduction

#### Dam failures

#### Artificial floods

#### Catastrophic events!



Kakhovka Dam failure (Ukraine), June 2023



**Derna** Dams failure (Libya), September 2023

# Introduction

#### Concrete dams





sudden collapse (partial or total) of the dam body



# Introduction

Triggering mechanisms



- Different initial phase
- Formation of a top
   breach in both cases
- Breach enlargement governed by the transport capacity of the outflow discharge

### Dam-breach numerical modelling

Aimed at:

- 1. Outflow hydrograph generation
- 2. Downstream flood wave propagation

...addressed separately

# Case study: Castagnara Dam (Calabria, Italy)



# Characteristics of the dam-reservoir system:

• Dam height:

 $Z_M = 85.5 \text{ m}$ 

Water volume stored up to the dam crest:  $W_M = 38 \text{ Mm}^3$ 

# Outflow hydrograph generation

Macchione (2008) simplified physically based model

#### $\rightarrow$ Main assumptions:

 $_{\odot}$  Shear stress-based erosion law dependent on parameter  $v_e$  = 0.07 m/s

 $\circ$  Triangular breach characterized by shape parameter tan  $\beta$ 





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#### Breach shape scenarios assumed for the case study

> Hypothetical failure simulated according to different breach shape scenarios: (1)  $\tan \beta = 0.2$ ; (2)  $\tan \beta = 0.5$ 



• Same released water volume: 38 Mm<sup>3</sup>

Different total volume of sediments (without porosity): 614218 m<sup>3</sup> vs 491309 m<sup>3</sup>

#### Goal

- > Evaluate the changes in bed elevation due to input sediment deposition
- Compare the results given by the assumed breach shape scenarios
- No data to calibrate the model
- > Sensitivity analysis of the results to the variability of some crucial parameters

Bedload transport formula  $\rightarrow$  Meyer-Peter and Muller (1948)

$$q_B = \psi 8(\theta - \theta_{cr})^{1.5} \sqrt{(s-1)gd^3}$$

$$\theta = \frac{h\sqrt{S_{fx}^2 + S_{fy}^2}}{(s-1)d} \longrightarrow \begin{array}{c} \text{Shields parameter} \\ \text{(dimensionless bed shear} \\ \text{stress)} \end{array}$$

 $\theta_{cr} = 0.047 \longrightarrow$ Critical value of the Shields parameter

 $s = \rho_s / \rho_w$   $\longrightarrow$  Sediment density coefficient

- d is the sediment diameter (average diameter of the Castagnara dam body: d = 0.05 m)
- $\psi$  is a pre-factor used to scale the bedload transport formula



Sensitivity analysis of the results to the Meyer-Peter and Muller pre-factor: MPM factor ( $\psi$ ) = 1, 2, 3, 4, 5, 7, 10

#### Case study location and potential flooded area



#### **Computational domain**

- From the dam toe to the seashore
- Area: 72.4 km<sup>2</sup>

#### Model set up



# 0 100 200 m String definitions Computational mesh

#### Set up of break lines and string definitions

- Break lines set up in order to create four regions:
  - 1. Thalweg region
  - 2. Main channel
  - 3. Floodplain
  - 4. Rest of the domain
- Set up of 29 string definitions (1 Inflow XS; 27 Output XS;
   1 Outflow XS)

#### **Computational mesh**

- 380582 cells
- Maximum area of cells: 100 m<sup>2</sup> (regions 1 and 2), 400 m<sup>2</sup> (region 3), 700 m<sup>2</sup> (region 4)

#### Results – <u>Temporal evolution of sediment deposition</u>



#### $Results - \underline{Final\ sediment\ distribution}$



#### Scenario 1

- No deposition at the Inflow XS
- Maximum deposition height of
  27.5 m given by MPM factor =1
- As MPM factor increases:
  - Deposition heights decrease
  - > The sediment is transported
    - more and more downstream

#### $Results-\underline{Final\ sediment\ distribution}$



#### Scenario 2

- Small depositions at the Inflow XS
- Maximum deposition height of
  - **25.5** m given by MPM factor =1
- As MPM factor increases:
  - Deposition heights decrease
  - > The sediment is transported

more and more downstream

Maximum deposition height

Average deposition height

#### Maximum deposition distance from the dam



#### MPM factor = 1t=0 bottom t=0 bottom t=0 bottom t=15 min water surface t=30 min water surface - t=60 min water surface t=30 min bottom t=15 min bottom t=60 min bottom t = 15 min $t = 30 \min$ $t = 60 \min$ MPM factor = 5t=0 bottom ···· t=0 bottom • t=0 bottom t=15 min water surface t=30 min water surface t=60 min water surface t=30 min bottom t=15 min bottom t=60 min bottom many m MPM factor = 10t=0 bottom t=0 bottom t=0 bottom t=30 min water surface t=15 min water surface t=60 min water surface t=30 min bottom t=15 min bottom t=60 min bottom Vary V

#### Results – **Bottom and water surface longitudinal profiles** – <u>Scenario 1</u>

#### Results – Bottom and water surface longitudinal profiles – <u>Scenario 2</u>



#### Results – <u>Final bottom</u> <u>longitudinal profiles</u>

Scenario 1 compared to Scenario 2

Results more sensitive to the MPM factor than to the breach shape scenario



# Thank you for listening!

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